Influence of the bed joint thickness on the bearing capacity of the brick masonry under compression loading: an ultrasound assessment

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Abstract
The aim of this paper is to analyse the influence of mortar bed joint thickness on the structural behaviour of brick-masonry specimens under compressive stress. Moreover, the progressive cracking of the material was studied during the loading process. Nine walls and nine columns were tested, combining three mortar bed joint thicknesses. Simultaneously, ultrasound, extensometer and Schmidt hammer measurements were recorded. The obtained results show that the thinner the mortar bed joint (5mm), the greater the breaking load (20.5N/mm²) and ultrasonic pulse speed. In addition, ultrasound evaluation shows good agreement with the extensometer measurements and allows accurate detection of the inner material failure.

Keywords: Non-destructive tests; ultrasound; brick masonry; compression stress; bed joint.

Introduction
Ceramic brick masonry has been used since ancient times in building and civil engineering construction, with a particularly long tradition in South America. The study of its structural behaviour and strength continues to focus the attention of researchers (Mohammed, 2011; Oliveira et al., 2006; Reyes, 2008; Reyes, 2009). This interest also meets the need to assess the load bearing capacity of historic structures such as bridges and buildings. The restoration of historic buildings, as well as the change of use of other well-preserved structures, requires assessment of their resistance in order to establish the needs of reinforcement or rehabilitation, based on usage requests (Lourenço, 2002; Chavez, 2005). Most of the ancient buildings are masonry constructions.

Furthermore, brick masonry is a heterogeneous and anisotropic material formed by bricks and mortar. Its behaviour up to failure is nonlinear and primarily depends both on its components (brick, mortar and brick-mortar interface) and its constructive disposition (bond and mortar bed joint thickness) (Senthivel, 2009). The normalised calculations available for determining masonry strength lead to significantly different values and neglect important aspects such as bed joint thickness (Monteagudo, 2010).

Intervention in the immovable cultural heritage is required to be as less aggressive as possible (Lombillo, 2007), with use of the non-destructive test (NDT) being an appropriate way to identify the properties of masonry structures. Among the possible NDTs used for mechanical characterisation, the ultrasonic pulse velocity (UPV) measurement is that successfully used in brick (Koroth, 1998) and stone masonries (Vasconcelos, 2008). Moreover, the technique has been also used in combination with other NDTs, such as thermography (Meola, 2005). However, when studying how the thicknesses of mortar bed joints affect the behaviour of brick masonry, the literature offers few results from experimental tests.

This paper analyses the influence of mortar bed joint thickness on the structural behaviour of brick masonry specimens under uniaxial compressive stress. In doing so, nine walls and columns have been tested that entail a combination of three mortar bed joint thicknesses, subjected to compressive load until failure. With the purpose of assessing the progressive cracking of the material during the loading process, NDT tests have been performed, such as UPV measurements. In addition, extensometer and Schmidt hammer measurements have been considered to verify the reliability of the UPV technique.

The first part of the present work was presented at the Spanish Fracture Conference held in 2011 (Monteagudo, 2011). The present paper includes new research advances and a more detailed and deeper discussion, based on the new experimental data. The evolution of the masonry specimens cracking by means of ultrasonic velocity assessment is also included.
Methodology

Design of the tests

Regarding the brick masonry columns and walls, three specimens were manufactured for each mortar bed joint thickness (5, 10 and 15mm). The dimension of the columns was 235x235x800mm, while the dimension of the walls was 750x115mm on plan view. The height of the walls varied between 650 and 830mm. The nomenclature used to designate the specimens is shown in Table 1.

<table>
<thead>
<tr>
<th>Joint thickness (mm)</th>
<th>Nomenclature</th>
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<tbody>
<tr>
<td>5</td>
<td>C 1.05</td>
</tr>
<tr>
<td>10</td>
<td>C 1.10</td>
</tr>
<tr>
<td>15</td>
<td>C 1.15</td>
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<table>
<thead>
<tr>
<th></th>
<th>C 2.05</th>
<th>C 2.10</th>
<th>C 2.15</th>
<th>C 3.05</th>
</tr>
</thead>
<tbody>
<tr>
<td>Walls</td>
<td>W 1.05</td>
<td>W 2.05</td>
<td>W 3.05</td>
<td></td>
</tr>
<tr>
<td>5</td>
<td>W 1.10</td>
<td>W 2.10</td>
<td>W 3.10</td>
<td></td>
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<tr>
<td>10</td>
<td>W 1.15</td>
<td>W 2.15</td>
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The specimens were subjected to the uniaxial compression test and, simultaneously, measurements of UPV, Schmidt hammer and extensometers were recorded.

Bricks

For the manufacture of the specimens, commercial clay bricks were used. The dimension of the bricks was 235x108x37mm. According to European Standard UNE-EN 772-1, the compressive strength of three brick specimens of 40x40x40mm was σc=59.8N/mm² (see Table 2).

Mortar

The mortar was made from Portland cement CEM I 42.5N type and standard silica sand with a maximum aggregate size of 1.5mm. The water-to-cement ratio was 0.5 and cement sand ratio was 1/6 (NBE FL-90, 1990). According to the European Standard UNE-EN 1015-11, (UNE-EN 1015-11, 2007), the compressive strength of three mortar specimens with dimensions 35x35x35mm was 22.9N/mm². Table 2 shows the compressive and flexion strength results of the brick and mortar specimens, as well as the average values.

<table>
<thead>
<tr>
<th>Specimen</th>
<th>Dimensions (mm)</th>
<th>Fc (kN)</th>
<th>σc (N/mm²)</th>
</tr>
</thead>
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<tr>
<td>brick</td>
<td>38x38x38</td>
<td>81.2</td>
<td>59.79</td>
</tr>
<tr>
<td></td>
<td>40x40x40</td>
<td>103.2</td>
<td></td>
</tr>
<tr>
<td></td>
<td>39x39x39</td>
<td>89.2</td>
<td></td>
</tr>
<tr>
<td>mortar</td>
<td>35x35x35</td>
<td>36.0</td>
<td>22.86</td>
</tr>
<tr>
<td></td>
<td>35x35x35</td>
<td>23.0</td>
<td></td>
</tr>
<tr>
<td></td>
<td>35x35x35</td>
<td>25.0</td>
<td></td>
</tr>
</tbody>
</table>

During the loading process, and for each load stage of 150kN, UPV measurements were taken simultaneously by means of Pundit ultrasound equipment with high frequency transducers (50-100kHz). In the case of the columns, UPV was measured in two directions (see Figure 3). In position one the wave passes through brick and mortar (B+M) and in position two through single brick (B). These measurements were taken in both X and Y-axis directions. As regards the walls, the measurement was carried out through single brick, without passing through...
mortar, as shown in Figure 4. Measurements were taken at three different heights in both the columns and the walls.

In addition, a Schmidt hammer was used to obtain an order of magnitude of the surface hardness variation on brick and mortar, during the loading of the columns. For this purpose, six measurements at different heights were taken.

**Figure 2.** Compressive strength test in walls. Source: self-elaboration.

**Results and discussion**

Figure 5.a shows the average UPV measurements taken in the columns depending on the applied load for the two aforementioned positions (see Figure 3) and three joint thicknesses. Figure 5.b shows similar results for the walls. Regarding the UPV in the columns, as shown in Figure 5.a, the average value is clearly lower when the wave passes through brick and mortar. The average UPV for the position 1 (B+M) was 3,863.2 m/s, while the average UPV for the position 2 (B) was 4,236.5 m/s. This is primarily due to the lower stiffness of the joint and presence of a discontinuity between the brick and the mortar. Before the maximum breaking load a slight increase in the UPV, for both the columns and walls, was observed. This phenomenon has been noted by various authors (Koroth, 1998), (Schuller, 1997). The increase is due to the greater compaction of the material that reduces the volume of voids. After exceeding the maximum breaking load, the appearance of cracks is shown by a sharp decrease in the UPV (Figures 5.a and 5.b). During continuous monitoring of structures, the decrease of ultrasonic pulse can be a clear indication of the appearance of internal cracks that may not have been manifested on the outside. Meglis et al. (Meglis, 2005) also observed a decrease in the average UPV as cracks propagated.

**Figure 3.** Scheme of the transducer position in the column UPV measurement. Source: self-elaboration.

**Figure 4.** Scheme of the UPV transducers in the walls. Source: self-elaboration.

**Figure 6.a** compares the average UPV of the columns for the various mortar bed joint thickness (5, 10 and 15mm) at each load stage. In addition, Figure 6.b shows similar results for the walls. For both the columns and walls, a clear relationship is seen between the UPV and the thickness of the mortar bed joint. The UPV measurement is higher in all the specimens with a lower mortar bed joint. Such an effect is due to the higher deformability of the mortar compared with that of the brick. Mortar deformation introduces tensile stresses that are transmitted to the brick through the mortar-brick interface. Moreover, the stiffness of the brick leads to a lower fracture load of the masonry.
The experimentally measured average strength of the specimens is shown in Table 3, in which the maximum rupture stresses for the columns and walls are listed. The individual values for each mortar bed joint thickness correspond to the average value of the three specimens tested. An increase in resistance, especially in the walls, is notable as the mortar bed joint thickness decreases. Lastly, the characteristic strength of the brick masonry has been obtained from the strength of its constituents. In order to obtain the resistance of the columns and the walls, among the experimental formulae available in the literature, that offered by Haller (1969) is widely accepted (see Eq. 1).

\[
R_M = \left(1 + 0.15R_t - 1\right)\left(8 + 0.057R_m\right)
\]

Where, \(R_t\) and \(R_m\) are the brick and mortar strength respectively and \(R_M\) is the masonry strength, all in kg/cm².

Through applying Eq. 1 to the experimentally values of Table 2 a wall strength of 17.9N/mm² was obtained. From the data presented in Table 3, it may be concluded that the formula offered by Haller is well suited in calculating the compressive strength of the columns, though it is not so suitable in calculating the compressive strength of the walls as it fails to consider wall slenderness. This equation could only be used in cases where the wall has a large thickness and in which, therefore, slenderness is lower.

In Table 4 \(f_k\) is the masonry characteristic strength, \(f_b\) and \(f_m\) are the normalized compressive strengths of the brick and the mortar respectively, all in N/mm², \(K\) is a constant value which varies between 0.5 to 0.6 in mm²/N depending on the presence of stitches, \(f_s\) is the average strength, \(f_j\) is the strength of each individual test, \(\delta\) is the standard deviation and \(n\) is the number of specimens tested.

The analytical values obtained by EC-6 and CTE allow the calculation of the characteristic compressive strength based on the compression strength of the constituent materials. Furthermore, this provides similar values to the experimental average obtained in the columns. Although the NBE FL-90 is repealed, it is included in the comparison since many brick buildings that are currently in service, have been calculated according to its requirements. The advantage provided by this standard is the use of experimental values to calculate the characteristic strength of the masonry. Should these preliminary tests not be made, the norms propose establishing of the characteristic strength of the wall depending on the type and individual strength of the constituent materials and mortar bed joint thickness. Once again, the accuracy of this standard is greater despite only walls being considered.

When design calculations proposed by the aforementioned standards are compared with the experimental results, the calculations of the projected masonry are, in all cases, somewhat conservative, especially when the mortar bed joint thickness is below 10mm. Despite the precautions taken, the lower bearing capacity of the walls, relative to that of the columns, is due to the slenderness and may induce an eccentricity of the load. By means of real scale specimens, Carpinteri et al. have verified that the breaking strength is inversely proportional to slenderness (Carpinteri, 2007).
Figure 7. UPV depending on the height, a) in C05 column and b) in W05 wall. Source: self-elaboration.

Figure 7.a shows the average UPV in the columns with a 5mm mortar bed joint for the two positions described in Figure 3: position 1 (B+M) and 2 (B), while Figure 7.b presents similar results for the walls with a mortar bed joint of 5mm. The obtained values for the columns and walls with mortar bed joint thicknesses of 10 and 15 mm showed similar results. In both figures the UPV is represented depending on the height and the moment in which the measure was taken (before, during or after the loading test). It is observed that the measurement of values depending on different heights shows slight variation, which confirms a uniform load distribution on the test specimens.

Figure 8 represents the average Schmidt hammer rebound index in the brick and mortar columns, depending on the various load stages. The rebound index is higher in the brick than in the mortar; this was expected due to the higher stiffness of the brick, as shown by the surface hardness. One notable variable is a tendency to increase the Schmidt hammer rebound index as the load increases that could be explained by the acquisition of compactness of the material. This increase is more pronounced in the mortar than in the brick, with the mortar being more deformable. Evaluation of the surface hardness confirms the findings obtained by the UPV measures as shown in Figure 8.

Moreover, the value of the UPV can be also indicative of the occurrence of internal cracks in the structure. As shown in Figure 9, there is a decrease in the Y-axis UPV measure for the specimen C1.05 from a load of 900N. This reduction corresponds to the appearance of a crack in the transverse direction (X-axis direction). Conversely, in the specimen C1.10 the crack appeared in the opposite direction (Y-axis) and, therefore, the UPV wave decreases in the X-axis direction. Finally, the specimen C3.15 has not produced any significant crack that interferes with the measures and, thus, the variation of the ultrasonic pulse is similar in each direction.

Figures 10.a and 10.b show stress-strain curves measured by means of extensometers in the columns and walls respectively. The stress-strain curves of the columns (Figure 10.a) show low experimental scattering and confirm a greater deformability of the columns with a thicker mortar bed joint. In the case of the walls (Figure 10.b) the results are similar, although there is a greater scattering attributable to wall slenderness and buckling problems (Sandoval, 2012).

Conclusions

Apart from the previously mentioned results, the conclusions to be drawn from this research are as follows:

From a methodological point of view non-destructive evaluation methods used during the experimental campaign offer reliable information on the deterioration and bearing status of the brick masonry. The continuing UPV measurement, complemented by the Schmidt hammer, are useful methods for assessing the structure without damaging the existing material. With proper calibration, the ultrasonic method can be useful, especially when the damaged areas are not detectable on visual inspection.

Regarding the compression strength results, there is a direct relationship between the thickness of the mortar bed joint and the bearing capacity of the masonry. As explained above, the smaller the thickness of the mortar bed joint the greater is the UPV registered.

The compressive strength of constituent materials determines the masonry strength; however, they alone do not allow
definitive conclusions about masonry compressive strength to be established. The models of the current standards establish the nominal strength depending on the constituent materials; nevertheless, aspects such as the thickness of the mortar bed joints are crucial in the nonlinear behaviour of brick masonry structures.

Acknowledgments

The authors wish to express their gratitude to the Ministerio de Economía and Competitividad for the grant DPI 2011-24876.

References


Figure 9. UPV depending on the load and scheme of the specimens rupture. Source: self-elaboration.